

The system shown in the figure is made up of a beam connected on its right end to a truss.

Data:

$$L_{EF} = L_{FG} = L_{EG} = 4 \text{ m}$$

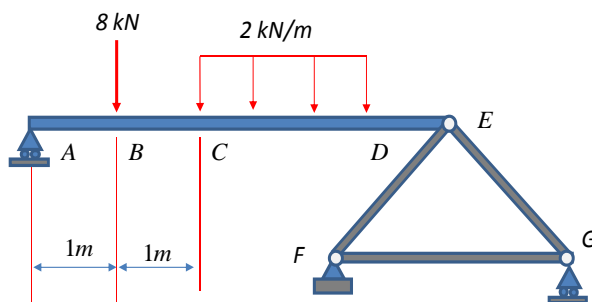
$$D_{EF} = D_{FG} = D_{EG} = 0,04 \text{ m (diameter)}$$

$$L_{AB} = L_{BC} = L_{DE} = 1 \text{ m}; L_{CD} = 2 \text{ m}$$

Young modulus = 210 GPA

Determine:

- DSI of the truss and its possible implications.
- Reaction forces in supports A and E.
- Reaction forces in supports F and G.
- Using the **method of the joints**, the axial forces in all members of the structure indicating if they are tensile or compressive.
- Using the **method of the sections**, the axial forces in bars FE and FG.
- Normal stress in member EF.



**Solution:**

In order to solve the exercise, we will separate both structures and add the corresponding internal forces as it is shown in the drawings.

### 1. DSI Isolated truss

$$EDSI = R - EDOF = 3 - 3 = 0$$

$$IL = 3.2. (2 - 1) = 6$$

$$IDOF = 3. (3 - 1) = 6$$

$$IDSI = IL - IDOF = 0$$

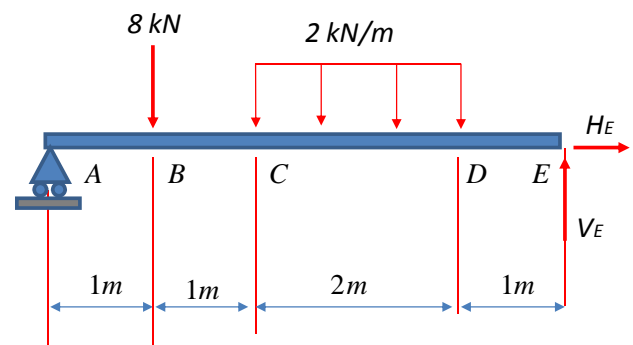
$$DSI = EDSI + IDSI = 0$$

Alternatively

$$DSI = r + m - 2j = 3 + 3 - 2.6 = 0$$

Thus, structure is entirely linked and the reactions exerted by the supports can be determined using equilibrium conditions.

### 2. Reaction forces beam



$$\sum M_E = 0 \rightarrow 5V_A - 4.8 - 2.2.2 = 0$$

$$V_A = 8 \text{ kN}$$

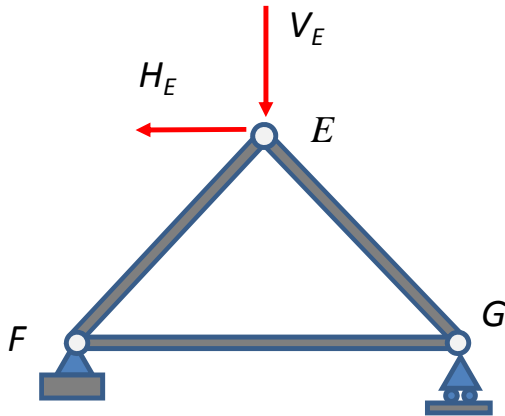
$$\sum F_y = 0 \rightarrow V_E = 2.2 + 12 - 8$$

$$V_E = 4 \text{ kN}$$

$$\sum F_x = 0 \rightarrow H_E = 0 \text{ kN}$$

### 3. Reaction forces in the truss

Isolating the system:



$$\sum M_G = 0 \rightarrow 4V_F - 2.4 = 0$$

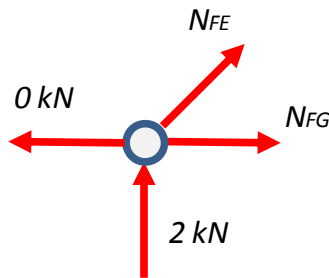
$$V_F = 2 \text{ kN}$$

$$\sum F_y = 0 \rightarrow V_G = 4 - 2 \rightarrow V_G = 2 \text{ kN}$$

$$\sum F_x = 0 \rightarrow H_F = 0 \text{ kN}$$

4. Internal forces (using the method of the nodes)

Node F



$$\sum F_y = 0 \rightarrow 2 + N_{FE} \cdot \sin 60 = 0$$

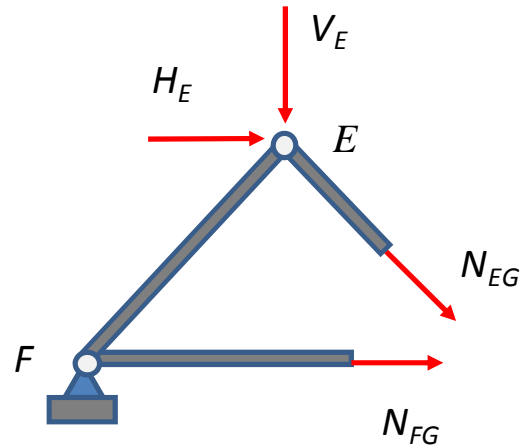
$$N_{FE} = -\frac{4}{3}\sqrt{3} \text{ kN (C)}$$

$$N_{GE} = -\frac{4}{3}\sqrt{3} \text{ kN (C) (simmetry)}$$

$$\sum F_x = 0 \rightarrow N_{FE} \cdot \cos 60 + N_{FG} = 0$$

$$N_{FG} = \frac{2\sqrt{3}}{3} \text{ kN (T)}$$

5. Internal forces (using the method of sections)



$$\tan 60^\circ = \frac{h}{2} \rightarrow h = 2\sqrt{3} \text{ m}$$

$$\sum M_E = 0 \rightarrow 2 \cdot V_F - N_{FG} \cdot 2\sqrt{3} = 0$$

$$N_{FG} = \frac{2\sqrt{3}}{3} \text{ kN (T)}$$

$$\sum F_y = 0 \rightarrow V_F - V_E - N_{EG} \cdot \cos 30 = 0$$

$$N_{FE} = -\frac{4}{3}\sqrt{3} \text{ kN (C)}$$

6. Calculation of the normal stress in EF.

$$\sigma = \frac{N}{A} = E \cdot \varepsilon = E \cdot \frac{\Delta l}{l} \rightarrow \Delta l = \frac{Nl}{EA}$$

$$\Delta l_{EF} = \frac{\frac{4}{3}\sqrt{3} \cdot 10^3 \text{ N} \cdot 4 \text{ m}}{210 \cdot 10^9 \text{ Pa} \cdot \pi \cdot 0.02^2 \text{ m}^2} = 0.035 \text{ mm}$$